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# 冬季输水梯形渠道冻胀时水力因素对刚性衬砌层内力影响研究

宋 玲, 陈瑞考, 马铭悦, 魏 鹏

(石河子大学 水利建筑工程学院, 新疆 石河子 832003)

**摘要:**对于冬季输水渠道,在其基土冻胀作用时边坡混凝土板力学模型得到实践工程检验的基础之上,又以实际工程渠道为参数,理论分析了在特定冻胀力作用类型及计算方法的条件下,其渠道坡角、水深和边坡混凝土板厚度的变化对边坡混凝土板内力的影响规律。结果表明:冬季输水渠道边坡板在冬季渠基土冻胀的过程中有沿坡面整体上移的趋势;在渠基土边坡整体稳定的前提条件下,增大渠道坡角、减小渠底宽、提高渠内水位可大大减少渠道冻胀作用时边坡板的内力,降低渠道冻胀破坏的机率;通过增加边坡板的厚度来提高渠道抗冻胀破坏能力是不可取的。该成果为季节冻土区冬季无冰盖输水渠道的抗冻胀破坏设计指出了设计新思路。根据设计流量,设计并调整渠道水力参数,尽可能使渠内水深提高,不仅降低渠道冻胀破坏的机率,还可以减少渠道用地面积和土方工程量。

**关键词:**渠道;冻胀破坏;衬砌层;冬季输水;内力

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## Influence of hydraulic factors on the inner forces of rigid lining of trapezoidal water conveyance canal in winter during frost heaving

SONG Ling, CHEN Ruikao, MA Mingyue, WEI Peng

(College of Water and Architectural Engineering, Shihezi University, Shihezi 832003, China)

**Abstract:** We tested in practical engineering the mechanical model of slope concrete slabs of winter water conveyance canals during the frost heaving of the bedsoil. Then we theoretically analyzed the influence of slope angle, water depth, and lining thickness on the inner forces of the rigid lining of the canal under the condition of a specific type of frost heaving force and calculation method. Results showed that the slope slab of the canal had a tendency to move upward along the slope during frost heaving of the bedsoil in winter. On the premise of overall stability of the slope, the inner forces and probability of canal damage due to frost heaving could both be greatly reduced by increasing slope angle, reducing canal bottom width, and rising water level. It is inadvisable to increase the slope slab thickness as a way to increase the frost heaving resistance capacity of the canal. The research findings can provide a new method for the anti-frost heaving design of water conveyance canals without ice cover in winter in seasonally frozen region. To design and adjust the hydraulic parameters of the canal so as to increase the water depth within the canal can reduce the probability of frost heaving damage to the canal, and can reduce the canal land area and earthwork volume.

**Key words:** canal; frost heaving damage; lining; water conveyance in winter; inner force

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作者简介: 宋 玲(1971-),女,河南人,副教授,博士,主要从事水工结构和冻土工程的研究。E-mail: xjsds10514@163.com

我国东北、西北、华北等广大地区,大部分为季节性冻土区。由于气温的季节性变化,季节性冻结和融化的土体对各类工程都有破坏作用,尤其是以水土为材料环境的各类水利工程建筑物冻害破坏最为严重。作为重要的输水建筑物,渠道过水使渠基土含水量增高,渠道的冻胀破坏可能性加大。渠道的冻胀破坏致使梯型混凝土等刚性衬砌渠道失去防渗功能。多年来,许多学者对渠基土冻胀及其对衬砌层破坏机理<sup>[1-2]</sup>、衬砌层受力分析<sup>[3-5]</sup>和力学模型<sup>[6-11]</sup>以及各种防冻胀破坏措施<sup>[12-15]</sup>、甚至提高材料抗冻融能力的新材料<sup>[16-20]</sup>等方面都进行了大量的研究。然而这些研究成果主要是针对负温期渠道,即渠基土全断面冻结膨胀的情况。学者们很少专门去研究冬季输水渠道的冻胀破坏问题,这是因为冬

季输水渠道的基土不像冬季停水渠道在整个横断面上全部冻胀,而是仅在渠内水位面以上边坡部位冻胀。加之冬季停水渠道研究成果确实也在冬季停水渠道上得到了检验和验证,所以人们一直认为:冬季输水渠道基土的冻胀不如全断面冻胀那么严重,渠道破坏程度也就比冬季停水渠道的轻。然而,实践证明,采用了防冻胀措施的渠道,在冬季输水的情况下,仍出现了冻胀破坏现象,如图1所示。图1中(a)至(d)为新疆玛纳斯河一级电站引水渠,该渠道于2006年建成,经过十年的运行,破坏虽不像无防冻胀措施的、运行时间较长的二级站图1(e)和四级站图1(f)引水渠出现纵向长裂缝且隆起破坏的那么严重,但仍有纵向长裂缝或局部翘起、裂缝等破坏现象。

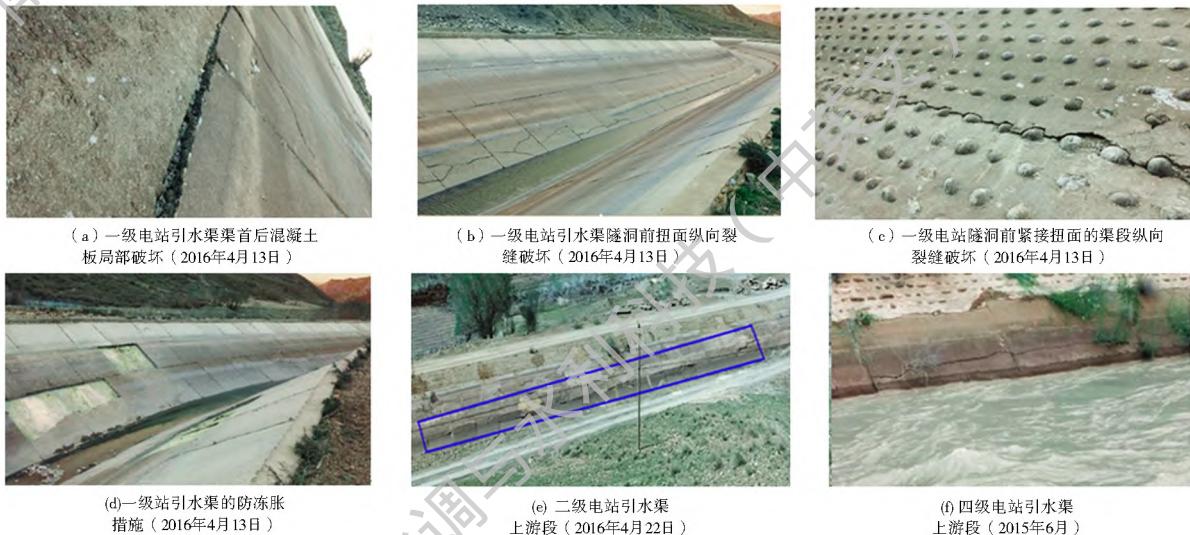


图1 新疆玛纳斯河电站引水渠冻胀破坏

Fig. 1 The frost damage of canals in power stations in Manasi river

据调查和研究成果来看,冬季停水渠道的冻胀破坏问题,目前已经得到了较好的解决。但对冬季输水渠道冻胀问题,专门研究的较少<sup>[21-25]</sup>。本文以相同土质条件、相同冻胀力强度和分布,采用相同衬砌层受力模型为前提,分析渠道横断面水力因素对衬砌层内力的影响,为相同条件下冬季输水渠道的设计及运行管理提供理论指导。

## 1 计算模型及参数

本文以新疆玛纳斯河四级电站引水渠为参照模型。四级电站引水渠全长4 153 km,渠道纵坡1/1400,边坡1:1.5,底宽3 m,渠深4.5 m。原设计衬砌材料为混凝土和浆砌卵石,经多年运行,边坡和底板破损严重,渗漏严重,在渠道渗漏水的作用下,部分沙土进入渠基表层,使其具有冻胀性,冻胀破坏严重,已经不能满足渠道的正常运行<sup>[26]</sup>。

该引水渠冬季输水,水面以下渠基土不发生冻胀,或仅有一小部分冻胀,衬砌层受力状态或力学模型与冬季停水渠道的完全不一样。冬季停水渠道不管是底板还是衬砌板均受冻胀力作用。而冬季输水渠道仅边坡板上作用有冻胀力。换言之,对于冬季输水渠道而言,渠道冻胀破坏就意味着渠边坡板的冻胀破坏。本文不考虑边坡板的纵向分缝,将边坡板视为底端固定铰支座、水位面处为活动铰支座的外伸梁,梁外伸部位作用冻胀力。边坡板的切向冻胀力作用形式(三角形分布)和强度值均相同,而法向冻胀力也是三角形分布,且按受力平衡得到的法向冻胀力强度值。该构件除了重力外,而边坡板水下部位作用有水压力、和未冻土体对其的摩阻力。

本文分析在上述力学模型的基础上研究渠内水深、坡角、边坡混凝土板厚度对边坡板内力影响规律。水深取8水平,依次取为0.5 m,1.0 m,1.5 m,

2.0 m, 2.5 m, 3.0 m, 3.5 m, 4.0 m; 坡角取  $15^\circ$ ,  $30^\circ$ ,  $45^\circ$ ,  $60^\circ$ ,  $75^\circ$ , 共 5 水平; 而边坡混凝土板厚度取 5 水平, 分别为 0.1 m, 0.2 m, 0.3 m, 0.4 m, 0.5 m。

## 2 结果分析

### 2.1 模型验证

四级电站引水渠冬季正常水深为 3.5 m, 多年平均冻结深度 1.5 m, 板厚为 0.15 m, 假设板与土的摩擦系数为 0.1, 则冬季渠道边坡板的轴力、弯矩、剪力计算结果见图 2。图中相对距离为截面至边坡板顶端的距离与边坡板斜长之比, 下文相关图中的横坐标轴名均与此相同。

由图 2(a) 可知, 边坡板轴力值随着距坡顶距离增大而先增大后减小, 即水面以上边坡板重力在坡面切向分力小于切向冻胀力, 而水面以下部位无冻胀力, 故轴力值也是降低的。边坡板在水面附近, 横截面的拉应力最大, 有拉断趋势。整个坡面的轴力

均为正值, 即边坡板受拉。值得注意的是: 在冻胀力发展过程中, 边坡板底端的轴力值由渠基土未冻结前的负值(边坡板与底板间相互挤压)逐渐增大, 甚至为正值。而其值根本上取决于冻胀力、板重和板土间摩阻力。由于渠底板与边坡板之间只能承受相互间的压力和摩阻力, 无法承受拉力。所以, 冬季输水渠道边坡板在渠基土冻胀过程中有沿坡面的上移与底板分离的破坏趋势。

由边坡板弯矩图 2(b) 可知, 边坡板上有负弯矩, 即在水面下某处为零, 该位置以上截面弯矩为负, 即边坡板的上表面受拉, 下表面受压, 朝渠内挠曲变形。而该位置以下弯矩为正, 即边坡板的上表面受压, 下表面受拉, 朝渠基土方向挠曲变形。因而, 边坡板在水面、距渠底约 1/3 斜长两个部位较易发生破坏。

由边坡板正截面的剪力, 图 2(c) 可知, 在水面附近出现剪力突变, 在此处最易出现剪断或发生相对的错动。

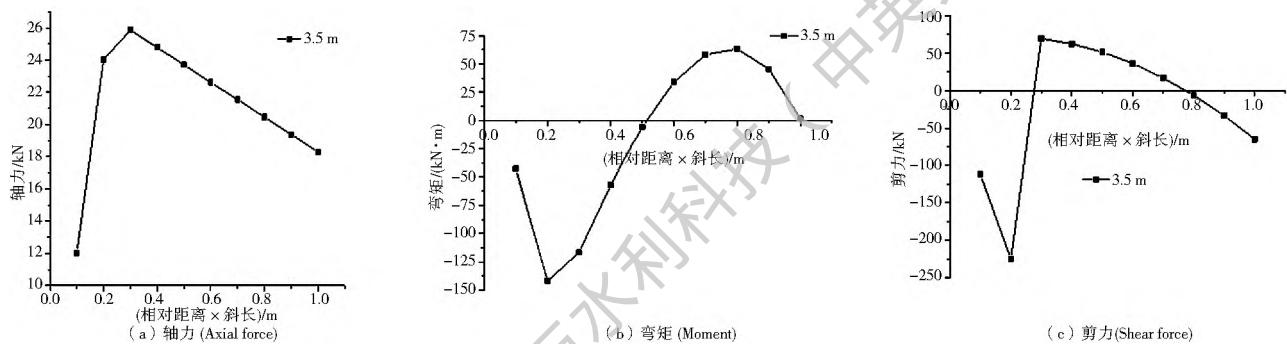


图 2 基土冻胀时边坡板的内力

Fig. 2 The inner forces of the slope lining of canal during frost heaving of bedsoil

综上所述, 边坡板易在水面附近最先发生破坏, 与前述图片中展示的破坏位置大体一致。从某种程度上来说, 本文所选用的衬砌板受力模型能够反应冬季输水情况下衬砌层受力情况。

### 2.2 坡角对边坡混凝土板内力的影响规律

冬季水深 3.5 m、板厚 0.1 m、板土间的摩阻系数 0.3 情况下, 坡角  $15^\circ \sim 75^\circ$  的边坡板内力随横截面位置变化见图 3。从图 3 中可以看出, 坡角增大, 边坡板

横截面的最大轴力值是减小的, 最大负弯矩值也是减小的, 并且板逐渐出现正、负弯矩共存的状态。同样边坡板剪力值随着坡角的增大也有相应关系。从这一变化规律来说, 在渠边坡基土整体稳定的前提下, 增大坡角可以降低渠基土的冻胀破坏作用。值得注意的是, 当冻土层厚度相对水面以上边坡板部位的长度较小时, 切向冻胀力值仅与冻深有关, 而板厚及板土间的摩阻系数取值均相同, 故图 3(a) 中有坡角

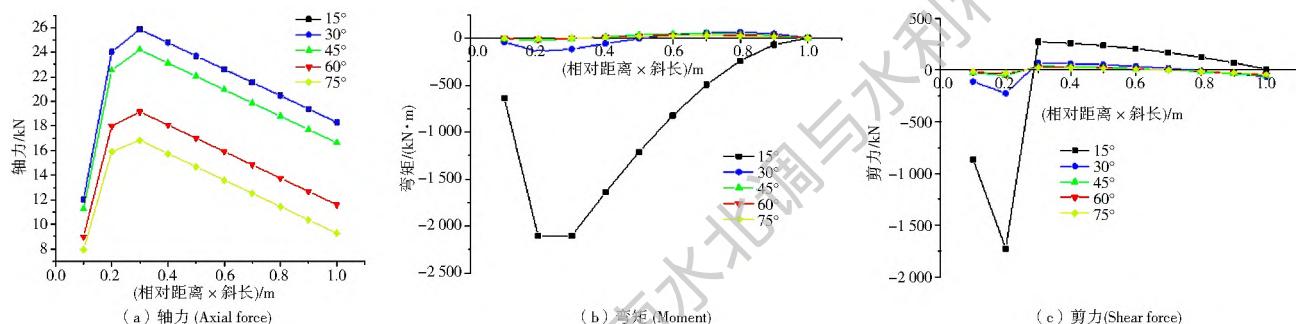


图 3 基土冻胀时, 不同坡角的边坡板内力随横截面位置关系

Fig. 3 Relation between inner forces of slope lining and position of cross section at different slope angles during frost heaving of bedsoil

15°和30°对应边坡板的轴力随横截面相对位置关系曲线相重合。由于边坡板下表面受到切向冻胀力强度相同,当坡角小时,相同水位下水压力对边坡板横截面产生的正弯矩值要远远大于坡角较大的,故有图3(b)所示坡角15°的负弯矩绝对值远远大于其它坡角的负弯矩绝对值,剪力值亦如此。

### 2.3 水深对边坡混凝土板内力的影响规律

坡角30°、边坡板厚0.1 m、板土间的摩阻系数0.3情况下,冬季水深0.5~4.0 m,边坡板各横截面内力随其位置变化关系见图4。从图4中可以看出,水深增大,最大轴力值总体上是增大的,但是水

深4.0 m 对应的最大值不符合增大的规律,这是因为冻结深度大于沿边坡方向的冻结长度,其它水深情况下总的切向冻胀力计算方法不再适用,进行了修正。最大正弯矩值是随着水深的增大而减小,出现负弯矩区的可能性也越来越大。最大剪力值随着水深的加大也有相应地减小。同时也可看出,渠内水位的变化对弯矩的影响较轴力的要大得多。从这一变化规律来说,尽可能增大冬季运行水深以减小渠基土的冻胀破坏作用。所以冬季输水渠道设计时,可设计小底宽的渠道,以保证在相同来水量情况下获得较大水深,从而有利于减少冻胀破坏作用。

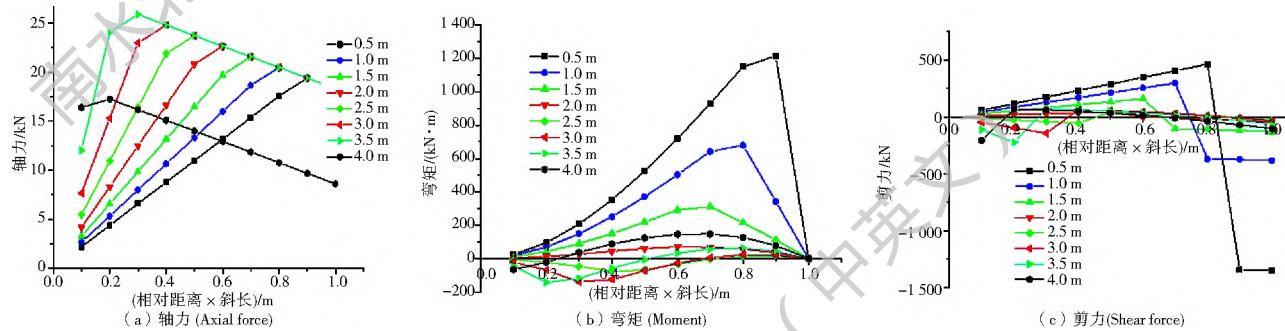


图4 基土冻胀时边坡板的内力随水深、正截面位置关系

Fig. 4 Relation between inner forces of slope lining and water depth or position of normal section during frost heaving of bedsoil

### 2.4 砌板厚对边坡混凝土板内力的影响规律

冬季渠水深0.5 m、坡角15°、摩阻系数0.3情况下厚度为0.1~0.5 m的边坡板内力随横截面位置的关系见图5,从图5中可以看出,板厚增大,最大轴力值是减小的,甚至可能出现压力。板厚增加,弯矩值

增大,但是最大值所在位置不变,仅与水深有关。而所受剪力最大值也随着板厚的增加而增大,发生突变的位置也不变,仅与水位有关。从这一变化规律来说,同一材料,增大板截面厚度,板的抗冻胀能力虽有所提升,但远不如最大弯矩值增长快。

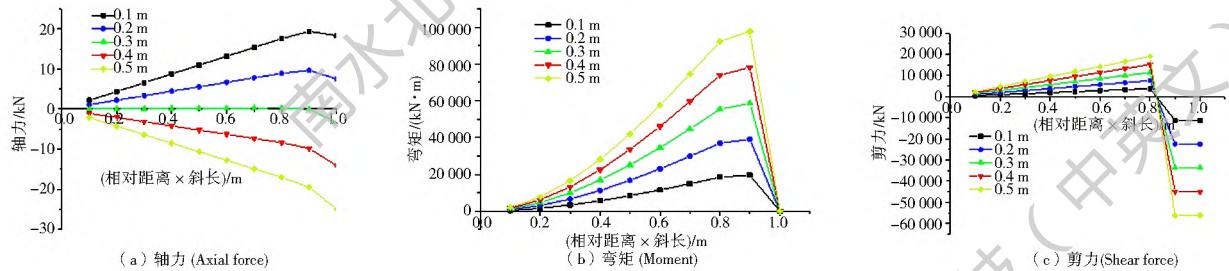


图5 基土冻胀时边坡板的内力随板厚、正截面位置关系

Fig. 5 Relation between inner forces of slope lining and lining thickness or position of normal section during frost heaving of bedsoil

## 3 结论与建议

(1) 从某种程度上说,所选用的力学模型能够较好地反映实际工程渠基土冻胀时边坡板的受力情况,理论分析的最危险位置也大体与工程的破坏位置一致。该力学模型可以指导季节冻土区的冬季输水渠道的抗冻胀设计。

(2) 渠基土冻胀作用时,相同的相对截面位置处的边坡板内力随着坡角的增大而减小,因此可以在

渠基边坡稳定的前提条件下增大坡角来削弱冻胀破坏效果。

(3) 其它条件相同时,渠水深越大,边坡板的内力越小。因而,综合考虑水力条件和施工要求,位于季节冻土区的冬季输水渠道渠底应尽量减小。

(4) 增加板厚,其它条件相同,边坡板的内力值也大大提高,因此,以增加板厚的方式来提高抗冻胀能力,既不经济,也达不到预期效果。因而可从提高材料的性能来提高抗冻胀能力。

(5) 在冻胀过程中,冬季输水渠道的边坡板有整体上移的趋势,且在水面附近最易发生拉裂破坏。

本文仅从抗冻胀的角度分析几个单因素对冬季输水渠道边坡混凝土衬砌板的内力影响,虽取得了一定的成果,可能与实际应用尚有一定的差别。此外,还需要进一步结合水力学知识,研究适用于具有抗冻胀能力性能的冬季输水渠道较经济断面。

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