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边坡植被对河道行洪影响研究

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摘要:为研究边坡植被对河道行洪的影响, 开展室内物理模型试验, 测试典型流量下边坡无植被、稀疏植被、稀疏植被+乔灌木和密集植被 4 种情形对应的水面线, 并根据原模型相似率反演试验工况对应的原型参数和水位差。结合经典的综合糙率计算公式进行数理分析, 构建边坡由无植被改为栽种植被引起综合糙率增加的计算公式, 提出分析边坡植被影响河道行洪的实用方法。结果表明: 综合糙率增加值与栽种植被区域湿周占总湿周的比值、栽种植被区域糙率值 2 个因素有关, 随栽种植被糙率值的增加而增加, 随河道宽深比的增加而减小; 当河道宽深比大于 60 时, 边坡栽种植被引起的综合糙率增加不超过 10%; 当河道宽深比大于 100 时, 边坡栽种植被引起的综合糙率增加不超过 7%。上述研究成果为边坡栽种植被河道的行洪设计与运行管理提供一定的参考。

关键词: 边坡植被; 河道行洪; 模型试验; 数理分析; 综合糙率

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行洪安全是河道规划、设计和治理关注的核心问题之一。考虑水土保持和环境保护等生态因素, 河道边坡一般种植草皮、小树, 或使用格宾网箱生态护坡, 网箱内密集种植草皮等植被, 以起到固持土壤、加固堤岸和减少水土流失的作用; 城市河道为满足人类景观、亲水等要求, 岸坡除种植草皮外, 还会栽种一些乔木、灌木等树木^[1]。边坡栽种植被对行洪能力和水力特性的复杂影响, 一般需要通过模型试验、理论分析或数值模拟确定, 以指导河道的规划设计和运行管理。

在理论分析方面的研究有: 许唯临^[2]推导了不需联立求解系数方程组的漫滩水流流速和床面切应力分布的计算公式以及床面平均切应力的解析计算式; 唐洪武等^[3]建立了等效综合曼宁糙率系数和等效植物附加曼宁系数的计算公式; 郑爽等^[4]分

析了植物挺立度、相对淹没高度及相对密植度对曼宁糙率的影响, 给出了淹没状态下含柔性水生植物河道的通用等效曼宁糙率系数经验公式。在植被影响方面的研究有: 程莉等^[5]通过数值模拟表明, 滩地植树区域的过流能力减小, 使主流流速增加, 对水位的影响较小; 黄本胜等^[6]对珠江三角洲窄滩复式河槽的研究也得到类似结论; 曾玉红等^[7]发现非淹没刚性植被的流动阻力与速度的平方成正比。其他学者研究发现: 在非平坦斜坡条件下, 茎盖度是影响曼宁粗糙度系数的主导因素^[8]; 植草高度 6、12 和 18 cm 情况下, 边坡糙率为 0.026~0.083^[9]; 渠底糙率随菹草高度的增加而增加, 随水位的降低而减小^[10]。刘慧博等^[11]对裸地进行处理后发现, 铺设枯枝物可以有效提高地表糙率系数值, 通过砾石压盖处理的地表糙率系数未有明显改变。针对河道原

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型糙率的率定,杨开林等^[12-14]、葛赛等^[15-16]提出了基于最小二乘法、支持向量机的系统辨识模型,给出了误差源和提高率定精度的方法。理论分析得到的切应力、附加糙率或等效糙率,揭示了栽种植被河道的水力特性,但需要相对较精确的试验条件或复杂的水力参数,而某些试验条件下得到的糙率结果,普适性未得到有效验证。因此,开展河工模型试验、测试典型流量下的过流特性是研究具体案例栽种植被对河道行洪影响的有效手段。

为研究边坡栽种植被对河道行洪的影响,开展典型河道的物理模型试验,以测试不同边坡植被的水面线和水位差,并反演对应原型河道的水力特征;结合经典的综合糙率计算公式,推导边坡栽种植被引起综合糙率变化的计算公式,给出分析行洪影响的计算方法,为河道规划、设计和治理提供理论依据和技术手段。

1 材料与方法

1.1 模型试验

试验研究在中国水利水电科学研究院水力学试

验厅进行。模型试验平台由进水系统、行洪河道、测量设备、调控设备和尾水系统5部分组成,见图1。进水系统包括地下水库、进水管、电泵、水平花管、稳流池和花墙。电泵由地下水库取水,经进水管、水平花管、稳流池和花墙后进入行洪河道。水平花管是一根两端封堵、沿河宽径向开口的水平管,水从下侧进入稳流池,经花墙进入河道,确保进流均匀。参照某河道的实地勘测资料,模型试验的行洪河道长度40 m、宽度8~12 m,试验水深4~8 cm、宽深比100~300。通过表面刮制0.6~0.7 cm的W形波纹凹槽,糙率值为0.018^[17-18]。测量设备包括电磁流量计和测针。电磁流量计安装在进水管上,用于监测上游来流量,精度为0.5%。测针用于监测典型位置的水位,精度为0.1 mm。沿线共布置6个水位测量断面,每个断面沿河宽方向布置5个测点,以其均值作为断面水位。流量和水位数据均为两名试验人员的比对校核值。控制设备包括进水管上的蝶阀以及河道下游的叠梁式尾门,分别用于控制上游来流量和下游水位。尾水通过1.5 m宽的渠道、地下廊道汇入地下水库。

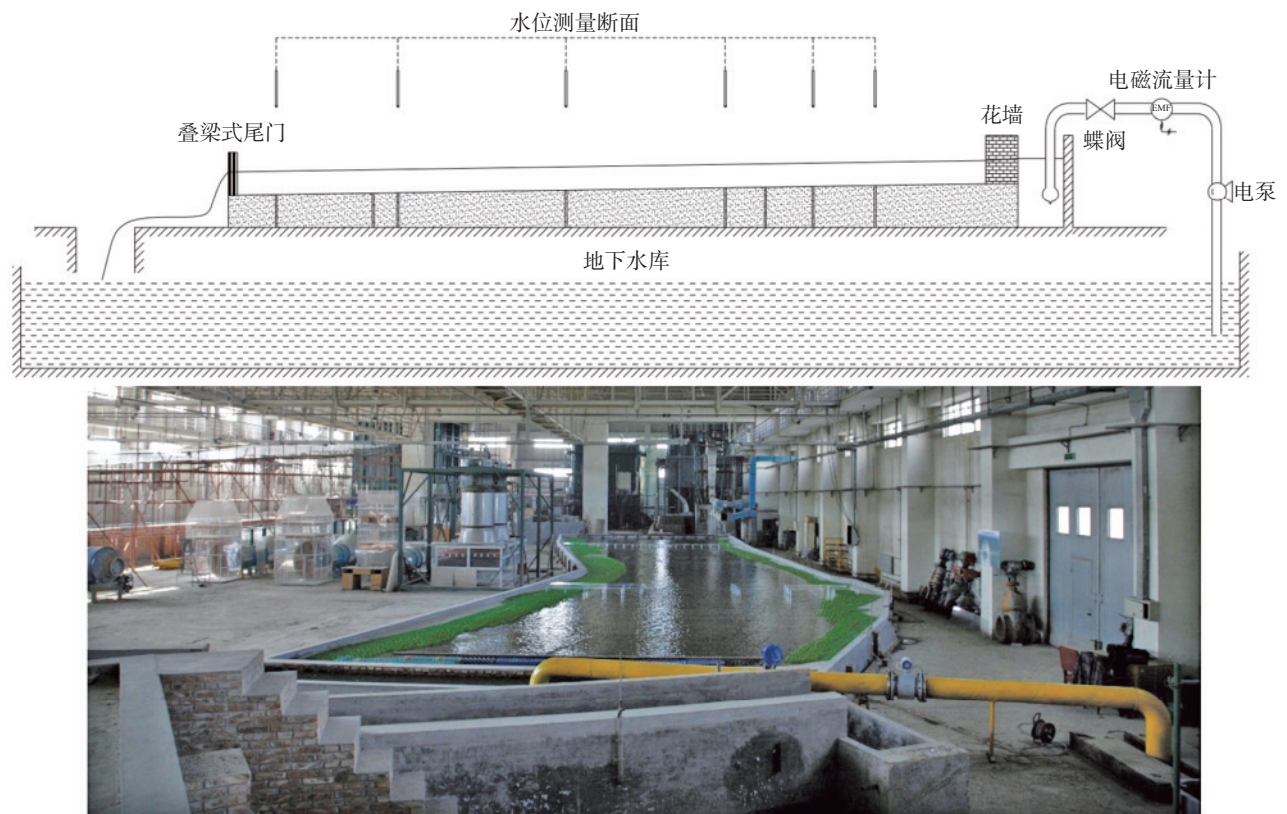


图1 模型试验平台

Fig. 1 Experimental platform

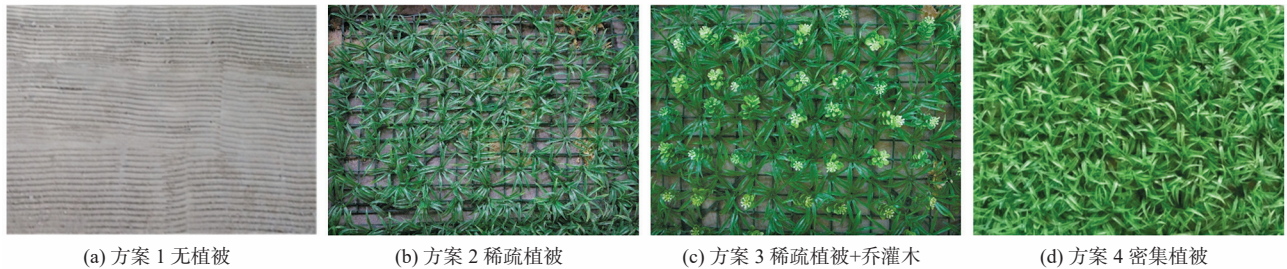
天然河道或城市河道的边坡植被通常有以下4种情形:河道边坡基本无植被,与河床组成和床面

特性一致;河道边坡长有稀疏的杂草、小树或种植矮小农作物;考虑滨河绿化、景观等因素,河道边坡

上侧种植深根系乔木、灌木,下侧种植草本植物;河道边坡采用网格护坡,在网格内密集种植马尼拉草、四季青等草类植物。

针对上述 4 种边坡植被情形,设置了 4 种试验方案,分别命名为无植被、稀疏植被、稀疏植被+乔灌木和密集植被。方案 1 的无植被,也就是边坡采用刮制 W 形波纹凹槽的加糙方式,与床面糙率相同,见图 2(a)。方案 2 的稀疏植被,采用塑料草模拟,塑料草的长度 25~40 mm,叶宽 1~6 mm,高度 10~15 mm,草间距 60 mm,见图 2(b)。方案 3 的稀疏植被+乔灌木,采用塑料草+塑料小花模拟,塑料

草的尺寸和间距与方案 2 相同,塑料小花杆径 3 mm、高度 50~60 mm、花间距 60 mm,1:55 几何相似的原型乔灌木杆径为 0.165 m、高度 2.75~3.30 m、间距 3.30 m,见图 2(c)。方案 4 的密集植被,采用塑料草模拟,塑料草的尺寸与方案 2 相同,草间距 30 mm,见图 2(d)。塑料草和塑料小花均扦插在塑料垫上,塑料垫网格尺寸为 30 mm×30 mm,见图 3。方案 2 仅在空心点位置扦插塑料草,方案 3 在空心点位置扦插塑料草、实心点位置扦插塑料小花,方案 4 在空心点和实心点位置均扦插塑料草。



(a) 方案 1 无植被 (b) 方案 2 稀疏植被 (c) 方案 3 稀疏植被+乔灌木 (d) 方案 4 密集植被

图 2 边坡植被情形
Fig. 2 Bank vegetation

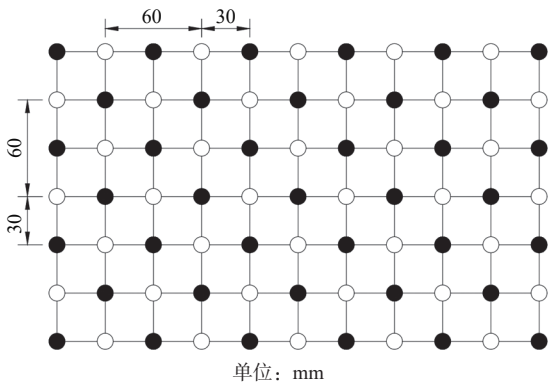


图 3 塑料垫网格

Fig. 3 Mesh grid of plastic piece

1.2 理论分析

综合糙率表示为不同糙率及其对应湿周的函数,计算公式^[19](PM 公式)为

$$n = \sqrt{\frac{\sum_{i=1}^N P_i n_i^2}{P}} \quad (1)$$

式中: P_i 为第 i 个分割面的湿周, m ; n_i 为第 i 个分割面的糙率; P 为整个断面的湿周, m ; N 为分割面的数量, 个。Einstein 等^[20]给出的函数关系式(EBM 公式)为

$$n = \left(\frac{\sum_{i=1}^N P_i n_i^{3/2}}{P} \right)^{2/3} \quad (2)$$

当 $N = 2$ 时,上述两式可统一为

$$n = \left(\frac{P_0 n_0^\alpha + P_1 n_1^\alpha}{P_0 + P_1} \right)^{1/\alpha} \quad (3)$$

式中: P_0 为河床湿周, m ; n_0 为河床糙率; α 为指数; P_1 为边坡湿周, m ; n_1 为边坡糙率。当 $\alpha = 2$ 时,上式即为 PM 公式;当 $\alpha = 1.5$ 时,上式即为 EBM 公式。

河床、边坡的湿周和糙率发生变化时,综合糙率变化的数学描述为

$$\Delta n = \frac{\partial n}{\partial n_0} \Delta n_0 + \frac{\partial n}{\partial P_0} \Delta P_0 + \frac{\partial n}{\partial n_1} \Delta n_1 + \frac{\partial n}{\partial P_1} \Delta P_1 \quad (4)$$

式中: $\frac{\partial n}{\partial n_0} = n \frac{P_0 n_0^{\alpha-1}}{P_0 n_0^\alpha + P_1 n_1^\alpha}$; $\frac{\partial n}{\partial P_0} = n \frac{P_1 (n_0^\alpha - n_1^\alpha)}{\alpha (P_0 + P_1) (P_0 n_0^\alpha + P_1 n_1^\alpha)}$;

$\frac{\partial n}{\partial n_1} = n \frac{P_1 n_1^{\alpha-1}}{P_0 n_0^\alpha + P_1 n_1^\alpha}$; $\frac{\partial n}{\partial P_1} = n \frac{P_0 (n_1^\alpha - n_0^\alpha)}{\alpha (P_0 + P_1) (P_0 n_0^\alpha + P_1 n_1^\alpha)}$ 。

式(4)整理得

$$\frac{\Delta n}{n} = \frac{P_0 n_0^\alpha}{P_0 n_0^\alpha + P_1 n_1^\alpha} \frac{\Delta n_0}{n_0} + \frac{P_0 P_1 (n_0^\alpha - n_1^\alpha)}{\alpha (P_0 + P_1) (P_0 n_0^\alpha + P_1 n_1^\alpha)} \frac{\Delta P_0}{P_0} + \frac{P_1 n_1^\alpha}{P_0 n_0^\alpha + P_1 n_1^\alpha} \frac{\Delta n_1}{n_1} + \frac{P_0 P_1 (n_1^\alpha - n_0^\alpha)}{\alpha (P_0 + P_1) (P_0 n_0^\alpha + P_1 n_1^\alpha)} \frac{\Delta P_1}{P_1} \quad (5)$$

当且仅当边坡植被发生变化时,式(5)转化为

$$\frac{\Delta n}{n} = \frac{P_1 n_1^\alpha}{P_0 n_0^\alpha + P_1 n_1^\alpha} \frac{\Delta n_1}{n_1} \quad (6)$$

若边坡初始状态无植被,即 $n_1 = n_0$,式(6)转化为

$$\frac{\Delta n}{n} = \frac{P_1}{P_0 + P_1} \frac{\Delta n_1}{n_1} = \frac{\lambda}{1 + \lambda} \frac{\Delta n_1}{n_1} \quad (7)$$

式中: $\lambda = P_1/P_0$ 。

2 结果与讨论

2.1 不同植被对水位差的影响

由图4可以看出:宣泄148 L/s流量(50年一遇流量)时,在下游水位相同的情况下,上游水位随植被密集程度的增加而增加,密集植被的水位壅高最明显,稀疏植被+乔灌木次之,稀疏植被壅高最小;在0~40 m范围内,种植密集植被使水位壅高0.3~1.3 mm,种植稀疏植被+乔灌木、稀疏植被使水位壅高0.1~0.4 mm。

按照1:10、1:25、1:40、1:55的模型比例尺,计算试验工况对应的原型参数,见表1。由图5可以看出:针对河床糙率为0.026的原型1,边坡植被

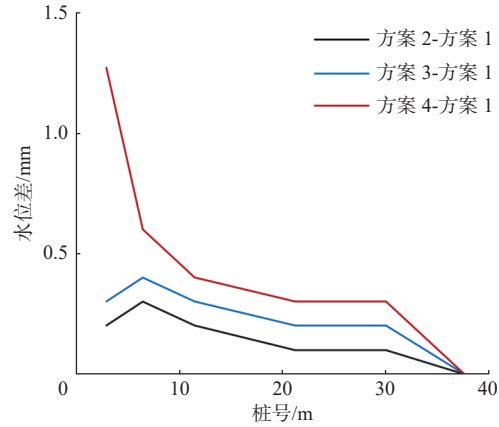


图4 模型试验水位差

Fig. 4 Water level difference in model experiments

表1 模型对应原型参数

模型	原型1	原型2	原型3	原型4	
比例尺	1:10	1:25	1:40	1:55	
长度/m	40	400	1600	2200	
宽度/m	8~12	80~120	200~300	320~480	440~60
水深/m	0.04~0.08	0.4~0.8	1.0~2.0	1.6~3.2	2.2~4.4
河床糙率	0.018	0.026	0.031	0.033	0.035

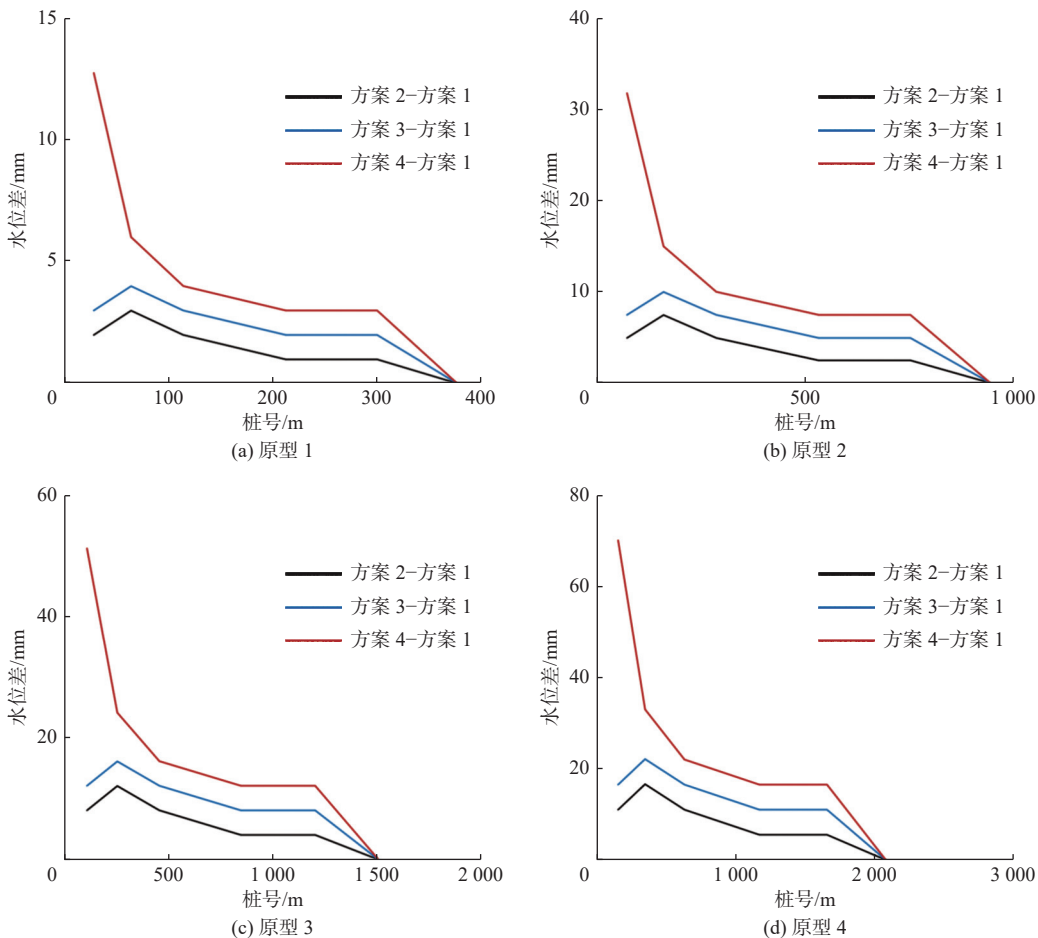


图5 原型河道水位差

Fig. 5 Water level difference in natural river

变化在 400 m 河道范围内引起的水位壅高为 2 ~ 15 mm; 针对河床糙率为 0.031 的原型 2, 边坡植被变化在 1 000 m 河道范围内引起的水位壅高为 5 ~ 35 mm; 针对河床糙率为 0.033 的原型 3, 边坡植被变化在 1 600 m 河道范围内引起的水位壅高为 8 ~ 50 mm; 针对河床糙率为 0.035 的原型 4, 边坡植被变化在 2 200 m 河道范围内引起的水位壅高为 11 ~ 70 mm。上述结果可为河道规划提供参照。

针对长江浦口段北岸 13.9 km 江滩上复绿工程, 北岸滩宽约 350 m、南岸滩宽约 150 m、平均槽宽 2500 m, 江滩复绿后引起的上游水位壅高为 5 ~ 10 mm^[21]; 对珠江三角洲河网的窄滩复式河道的研究表明, 滩地种树后引起的水位壅高在 100 mm 以下^[6]; 赣抚平原灌区抚河故道与无任何植被相比, 平水年的水位壅高值为 90 mm^[22]。这说明模型试验结果与其他成果具有较好的一致性。因此, 对于边坡植被区域占总过水区域 10% 以下的宽浅型河道, 设

计时考虑 10 cm 的水位壅高是基本可行的。

2.2 植被对综合糙率的影响

常见河床糙率的变化范围为 0.020 ~ 0.040, 滩地糙率的变化范围为 0.040 ~ 0.090^[23]。因此研究河床糙率为 0.020、0.025、…、0.040, 边坡糙率增加为 0.040、0.045、…、0.090, 不同宽深比河道的综合糙率变化。

由图 6 可以看出: 对于宽深比为 20、40、60 和 100 的河道, 综合糙率增幅随边坡糙率的增加而增加, 随河床糙率的增加而减小; 对于宽深比为 20 的河道, 当河床糙率为 0.020 时, 边坡糙率增加为 0.040 ~ 0.090 引起综合糙率增加 10% ~ 30%; 当河道宽深比大于 60 时边坡糙率增加引起的综合糙率增加不超过 10%, 当河道宽深比大于 100 时边坡糙率增加引起的综合糙率增加不超过 7%; 若河床糙率大于 0.025, 则边坡糙率引起的综合糙率增加不超过 5%。

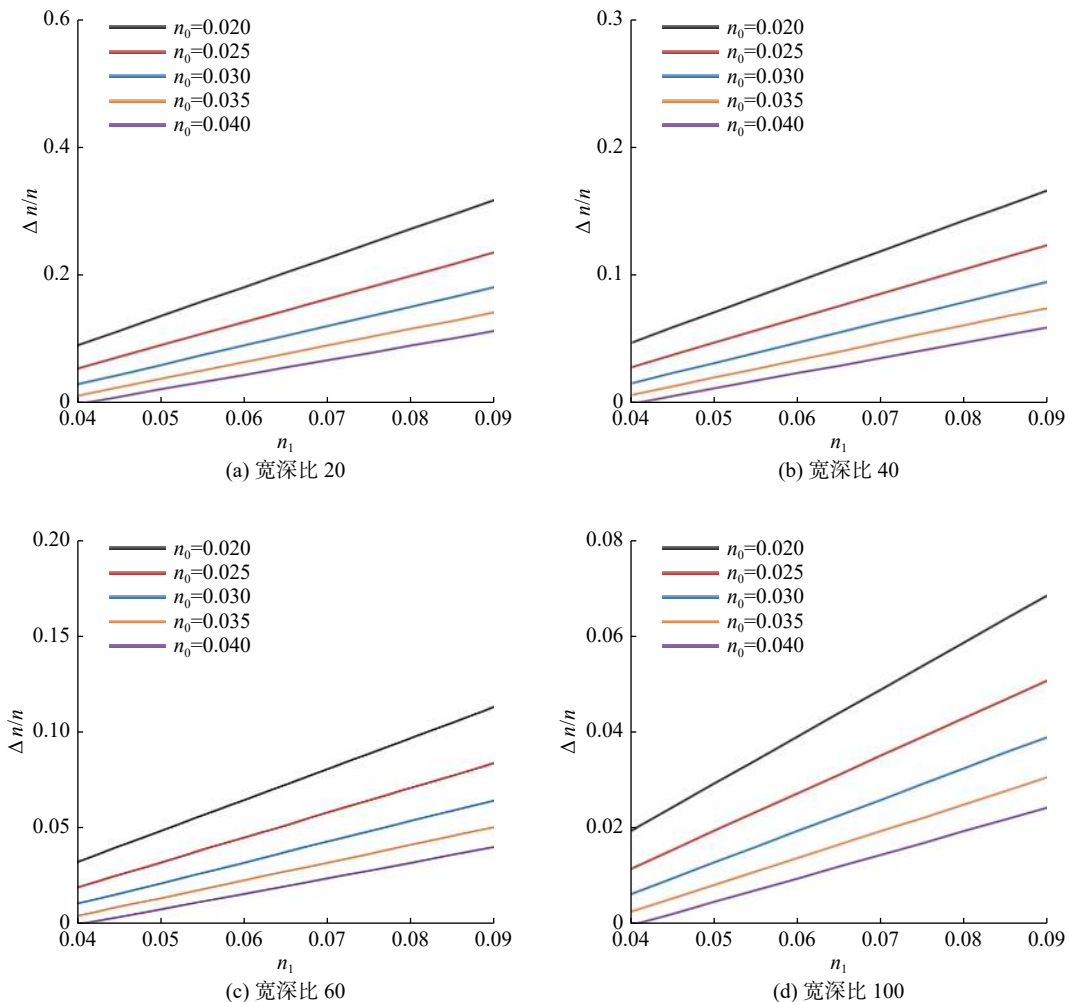


图 6 综合糙率随边坡糙率变化

Fig. 6 Composite roughness versus bank roughness

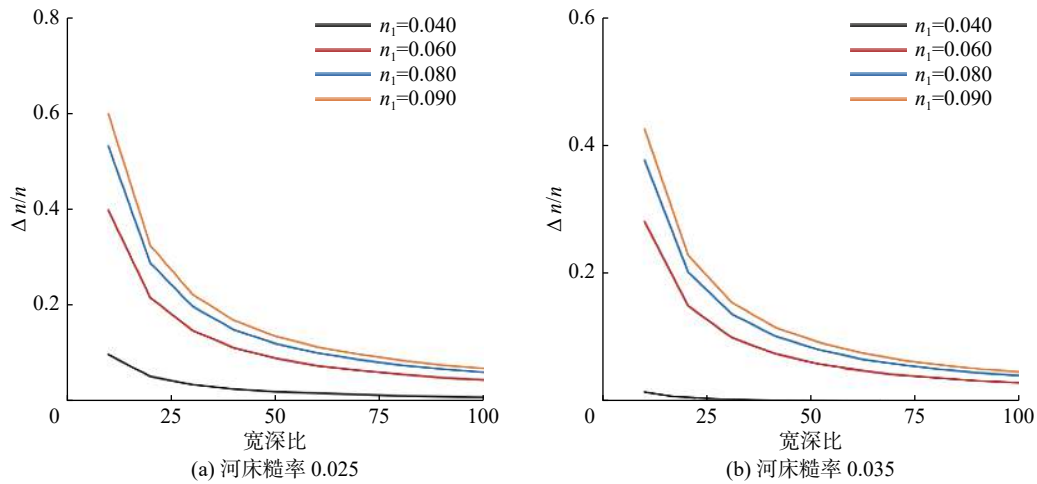


图7 综合糙率随宽深比变化

Fig. 7 Composite roughness versus width-depth ratio

由图7可以看出:对于河床糙率为0.025和0.035的河道,综合糙率变化的相对值随宽深比的增加而减小;当宽深比小于30时,边坡糙率增加引起的综合糙率增加较明显。

2.3 植被影响的设计分析

工程设计时,边坡植被对河道行洪影响可采用如下方法进行分析。

根据河道的床面特征和组成,以及拟栽种植被的种类、高度和密度等,查阅工程设计手册,确定河床的初始糙率 n_1 和由于栽种植被引起的糙率增量 Δn_1 。

根据设计资料或进行恒定均匀流计算,确定拟栽种植被区域与未栽种区域的湿周比值,即 λ 。恒定均匀流计算公式为

$$Q = AC\sqrt{Ri} \quad (8)$$

式中: Q 为设计流量, m^3/s ; A 为过水断面面积, m^2 ; $C = \frac{1}{n}R^{1/6}$; R 为水力半径, m ; i 为水力坡降。

根据式(7),计算综合糙率的增量 Δn ;根据式(8)计算边坡栽种植被后的水深,分析栽种植被对河道行洪的影响。

以实地勘测典型河道为例,底宽350 m、边坡1:10、底坡0.002、河床糙率0.035。50年一遇设计流量为 $3325 \text{ m}^3/\text{s}$,根据式(8)计算的恒定均匀流水深为3.26 m;20年一遇设计流量为 $1836 \text{ m}^3/\text{s}$,恒定均匀流水深为2.30 m。在20年一遇水面线和50年一遇水面线之间的河道边坡,拟栽种植被,设计糙率为0.065,与未栽种区域的湿周比值 λ 为0.052,代入式(7)得边坡栽种植被引起的综合糙率增量为0.0015,宣泄50年一遇洪水的水深为3.34 m,

水深增加0.08 m,与原型4的试验结果是一致的。利用提出的方法,对文献[24]中梯形河道边坡粘贴不同草皮和护岸的综合糙率进行计算,得到3种草皮和护岸的平均综合糙率分别为0.019、0.021和0.025,与模型试验得到的0.019、0.022和0.026基本一致。对模型试验结果和文献资料的复核说明,提出的设计分析方法具有一定的可行性和实用性。

3 结论

对典型河道开展物理模型试验,测试无植被、稀疏植被、稀疏植被+乔灌木和密集植被4种边坡的水力特性。在0~40 m范围内,种植密集植被使水位壅高0.3~1.3 mm,种植稀疏植被+乔灌木、稀疏植被使水位壅高0.1~0.4 mm。根据模型相似理论,反演了比例尺为1:10、1:25、1:40、1:55对应原型河道的水力特性,结果可为河道规划提供参照。

对综合糙率PM公式和EBM公式进行数理分析,构建边坡由无植被改为栽种植被引起综合糙率增加的计算公式。综合糙率变化主要与栽种植被区域湿周占总湿周的比值及栽种植被区域糙率值有关。综合糙率增量随栽种植被区域糙率的增加而增加,随河道宽深比的增加而减小。当河道宽深比大于60时,边坡栽种植被引起的综合糙率增加不超过10%;当宽深比大于100时,引起的综合糙率增加不超过7%。该研究可为河道规划、设计和治理提供快速、实用的技术手段。

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Study on influence of bank vegetation on river flood discharge

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Abstract: Urban river channel is an important element in city economy and ecology development. Considering soil

and water conservation, landscaping or ecological factors, the river bank is usually planted with grass, shrub and other vegetation. The plant coverage has changed the flow surface of the river bank, as well as the flood discharge capacity. Flood control and management is the most crucial factor for the planning of urban rivers, thus, it is quite necessary to study the influence of bank vegetation on river flood discharge. In order to analyze the effect of bank vegetation on river flood discharge, physical model experiments were carried out in laboratory. An experimental platform was built with the length of 40 m, the width of 8 to 12 m, the water depth of 4 to 8 cm and the width depth ratio of 100 to 300. The water levels of no vegetation, sparse vegetation, sparse vegetation + shrub and dense vegetation, were tested under typical flow. Then, the prototype parameters and water level difference corresponding to the tested conditions, were deduced according to the similarity principle. Mathematical analysis was further conducted on basis of the classical composite roughness calculation formulas, including Pavlovskij formula and Einstein-Banks formula. An equation was proposed to determine the increase of composite roughness while the river bank changed from no vegetation state to planted vegetation state. A practical procedure was also given to analyze the influence of bank vegetation on river flood discharge. The results showed that, the water level of the tested channel increased by 0.3 to 1.3 mm due to the dense vegetation, while it increased by 0.1 to 0.4 mm due to the sparse vegetation and sparse vegetation + shrub. The denser the bank vegetation, the higher the water level. For a river with the length of 400 m, the water depth of 0.4 to 0.8 m, and the roughness of 0.026, the water level increased by 2 to 15 mm. For a river with the length of 2200 m, the water depth of 2.2 to 4.4 m, and the roughness of 0.035, the water level increase could be large as 70 mm. When the river bank planted vegetation, the increase of composite roughness was related to two factors, namely, the ratio of the covered wetted perimeter to the total wetted perimeter and the roughness of the covered area. It increased with the growth of the roughness of the vegetation covered area, and decreased with the increase of width-depth ratio. While the width-depth ratio was larger than 60 and 100, the growth of composite roughness due to planted vegetation was no more than 10% and 7%, respectively. Urban river bank is usually planted with grass, shrub, or other vegetation, and the water level increases with the growth of the vegetation density. The water level growth can be large as 70 mm in the river length of 2200 m. The water level increase should be given special consideration. The composite roughness increase is related to the ratio of the planted wetted perimeter to the total and the roughness of the planted area. For the river whose width-depth ratio is larger than 100, the increase of composite roughness is no more than 7%. The formula was proposed to calculate the variation of composite roughness while the river bank vegetation changed. Further, an analysis procedure was given to guide further design.

Key words: bank vegetation; river flood discharge; model experiment; mathematical analysis; composite roughness